CHAPTER 1

Problem 1:

(a) Trapezoidal channel with side slopes m_1 and m_2

$$A = (b + m_1 \frac{y}{2} + m_2 \frac{y}{2})y$$

$$T = b + y(m_1 + m_2)$$

$$P = b + y\sqrt{1 + m_1^2} + y\sqrt{1 + m_2^2}$$

$$D = A/T$$

$$R = A/P$$

- (b) Trapezoidal channel with one vertical side Set $m_1 = 0$ and $m_2 = m$ in the equations given in part (a).
- (c) Right triangular channel Set $m_1 = 0$, b = 0, and $m_2 = m$ in the equations given in part (a).

Problem 2:

(a) From the problem statement:

$$\gamma = 62.4 \text{ lbf/ft}^3$$

 $Y_c = \text{depth to centroid} = 2 \text{ ft}$
 $A = \text{d} (1 \text{ ft/ft}) = 4 \text{ ft} (1 \text{ ft/ft}) = 4 \text{ ft}^2/\text{ft}$

By using Equation 1.9
$$F_p = \gamma Y_c A = 62.4(2)(4) = 499 \, lbf / foot$$

The e hydrostatic pressure force is normal to the vertical sidewall.

(b) From the problem statement:

$$\gamma = 62.4 \ lbf/ft^3$$
 $Y_c = \text{depth to centroid} = 2 \ ft$
 $A = (d^2 + (\text{md})^2)^{1/2} (1 \ ft/ft) = (4^2 + (2x4)^2)^{1/2} \ ft (1 \ ft/ft) = 8.94 \ ft^2/ft$

By using Equation 1.9 $F_p = \gamma Y_c A = 62.4(2)(4) = 1116 \, lbf / foot$

The hydrostatic pressure force is normal to the inclined side.

Problem 3:

$$v = 2.5v_* \ln \left(\frac{30z}{k_s} \right)$$

Let $k = k_s/30$. Then

$$v = 2.5v_* \ln \left(\frac{z}{k}\right)$$

Define q= discharge per unit width. By definition (see Equation 1.2)

$$q = \int_{k}^{y} v dz = \int_{k}^{y} \left[2.5v_{*} \ln \left(\frac{z}{k} \right) \right] dz = \int_{k}^{y} \left[2.5v_{*} \ln z \right] dz - \int_{k}^{y} \left[2.5v_{*} \ln k \right] dz$$

$$q = 2.5v_{*} \left[z \ln z - z \right]_{k}^{y} - 2.5v_{*} \left[z \ln k \right]_{k}^{y} = 2.5v_{*} \left[y \ln \frac{y}{k} - (y - k) \right]$$

Problem 4:

Noting that q= discharge per unit width and $k=k_s/30$ as in Problem 3, by definition (see Equation 1.3)

$$V = \frac{q}{y - k} = 2.5v_* \left[\frac{y \ln \frac{y}{k} - (y - k)}{y - k} \right] = 2.5v_* \left[\frac{y \ln \frac{y}{k}}{y - k} - 1 \right]$$

Because y >> k, we have $(y-k) \approx y$. Therefore

$$V = 2.5v_* \left[\ln \frac{y}{k} - 1 \right]$$

Problem 5:

For the velocity distribution given, obviously, the velocity is maximum at the free surface. Substituting z = y in

$$v = 2.5v_* \ln \left(\frac{30z}{k_s} \right)$$

we obtain

$$v_{\text{max}} = 2.5v_* \ln \left(\frac{30y}{k_s} \right) = 2.5v_* \ln \left(\frac{y}{k} \right)$$

with $k = k_s/30$.

Problem 6:

Using the results of Problems 4 and 5

$$\frac{v_{\text{max}}}{V} - 1 = \frac{2.5v_* \ln\left(\frac{y}{k}\right)}{2.5v_* \left[\ln\frac{y}{k} - 1\right]} - 1 = \frac{1}{\ln\frac{y}{k} - 1}$$

with $k = k_s/30$

Problem 7:

We found an expression for V in Problem 4. Here we will determine the value of z for which v=V. In other words with $k=k_s/30$

$$2.5v_* \ln \left(\frac{z}{k}\right) = 2.5v_* \left[\ln \frac{y}{k} - 1\right]$$

$$\ln\left(\frac{z}{k}\right) = \left[\ln\frac{y}{k} - 1\right] = \ln\left(\frac{y}{k}\right) - \ln(2.718) = \ln\left(\frac{y/k}{2.718}\right)$$

and

$$z = \frac{y}{2.718} = 0.37y$$

The point velocity at distance 0.37y from the bottom or 0.63y from the free surface will be equal to the cross sectional average velocity. Therefore, the velocity measured at 0.6y from the surface will be a good approximation to the average velocity.

Problem 8:

With $k = k_s/30$, we have

$$v = 2.5v_* \ln \left(\frac{z}{k}\right)$$

By definition

$$\beta = \frac{1}{V^2 A} \int v^2 dA$$

Using the velocity distribution given, for unit width, we can write

$$\beta = \frac{(2.5v_*)^2}{V^2(y-k)} \int_k^y \ln^2\left(\frac{z}{k}\right) dz$$

Let us first evaluate the integration

$$\int_{k}^{y} \ln^{2} \left(\frac{z}{k}\right) dz = \int_{k}^{y} (\ln z - \ln k) (\ln z - \ln k) dz = \int_{k}^{y} (\ln^{2} z - 2 \ln z \ln k + \ln^{2} k) dz$$

$$\int_{k}^{y} \ln^{2} \left(\frac{z}{k}\right) dz = \left[z \ln^{2} z\right]_{k}^{y} - 2 \int_{k}^{y} \ln z dz - 2 \ln k \int_{k}^{y} \ln z dz + \ln^{2} k \int_{k}^{y} dz$$

$$\int_{k}^{y} \ln^{2} \left(\frac{z}{k} \right) dz = \left[z \ln^{2} z - 2(z \ln z - z) - 2 \ln k (z \ln z - z) + \ln^{2} kz \right]_{k}^{y}$$

$$\int_{k}^{y} \ln^{2}\left(\frac{z}{k}\right) dz = 2(y-k) + y \ln^{2}\left(\frac{y}{k}\right) - 2y \ln\left(\frac{y}{k}\right) \tag{*}$$

Also from Problem 4

$$V = 2.5v_* \left[\ln \frac{y}{k} - 1 \right]$$

$$V^{2} = (2.5v_{*})^{2} \left[\ln \frac{y}{k} - 1 \right]^{2} = (2.5v_{*})^{2} \left[\ln^{2} \frac{y}{k} - 2 \ln \frac{y}{k} + 1 \right]$$

With y >> k and $(y-k) \approx y$

$$V^{2}(y-k) = (2.5v_{*})^{2} y \left[\ln^{2} \frac{y}{k} - 2 \ln \frac{y}{k} + 1 \right]$$
 (**)

Substituting Equations (*) and (**) into the expression for β

$$\beta = \frac{\left[(2.5v_*)^2 \right] 2(y-k) + y \ln^2(\frac{y}{k}) - 2y \ln(\frac{y}{k})}{(2.5v_*)^2 y \left[\ln^2 \frac{y}{k} - 2 \ln \frac{y}{k} + 1 \right]}$$

Noting $(y-k)\approx y$ and simplifying

$$\beta = \frac{y + y \left[1 + \ln^2(\frac{y}{k}) - 2\ln(\frac{y}{k}) \right]}{y \left[\ln^2 \frac{y}{k} - 2\ln \frac{y}{k} + 1 \right]} = 1 + \frac{1}{\left[\ln \frac{y}{k} - 1 \right]^2}$$

From Problem 6

$$\frac{v_{\text{max}}}{V} - 1 = \frac{1}{\ln \frac{y}{k} - 1}$$

Substituting this into the expression for β and rearranging

$$\beta = 1 + \left[\frac{v_{\text{max}}}{V} - 1 \right]^2$$

Problem 9:

$$v_* = \left(\frac{\tau_0}{\rho}\right)^{1/2} = \left(\frac{3.7}{1000}\right)^{1/2} = 0.061 m/s$$

From Problem 4

$$V = 2.5v_* \left[\ln \frac{y}{k} - 1 \right] = 2.5v_* \left[\ln \frac{30y}{k_s} - 1 \right] = (2.5)(0.061) \left[\ln \frac{30(0.94)}{(0.001)} - 1 \right] = 1.41m/s$$

$$q = \left[y - \frac{k_s}{30} \right] V \approx yV = (0.94)(1.41) = 1.33m^2/s$$

From Problem 5

$$\frac{v_{\text{max}}}{V} - 1 = \frac{1}{\ln \frac{30y}{k_s} - 1} = \frac{1}{\ln \frac{30(0.94)}{(0.001)} - 1} = 0.11$$

$$\beta = 1 + \left[\frac{v_{\text{max}}}{V} - 1 \right]^2 = 1 + (0.11)^2 = 1.01$$

Rate of momentum transfer = $\beta \rho q V = (1.01)(1000 \frac{kg}{m^3})(1.33 \frac{m^2}{s})(1.41 \frac{m}{s}) = 1894 \frac{kgm}{s^2} / m$

$$\alpha = 1 + 3 \left[\frac{v_{\text{max}}}{V} - 1 \right]^2 - 2 \left[\frac{v_{\text{max}}}{V} - 1 \right]^3 = 1 + 3(0.11)^2 - 2(0.11)^3 = 1.03$$

Rate of kinetic transfer = $\alpha \frac{\rho}{2} qV^2 = (1.03) \frac{1}{2} (1000 \frac{kg}{m^3}) (1.33 \frac{m^2}{s}) (1.41 \frac{m}{s})^2 = 1362 \frac{Nm}{s} / m$

Problem 10:

$$A_1 = 1 m \times 20 m = 20 m^2$$

 $A_2 = 4 m \times 4 m = 16 m^2$
 $A_3 = 0.5(1 m \times 22 m) = 11 m^2$

$$Q = V_1 A_1 + V_2 A_2 + V_3 A_3 = 0.5(20) + 1.5(16) + 0.3(11) = 37.3 \ m^3 / s$$

$$V = \frac{Q}{A} = \frac{Q}{A_1 + A_2 + A_3} = \frac{37.3}{20 + 16 + 11} = 0.79 \ m / s$$

Problem 11:

From Equation 1.15

$$\beta = \frac{V_1^2 A_1 + V_2^2 A_2 + V_3^2 A_3}{V^2 A} = \frac{0.5^2 (20) + 1.5^2 (16) + 0.3^2 (11)}{0.79^2 (47)} = 1.43$$

Then by using Equation 1.11 with $\rho = 1000 \ kg/n$

Rate of momentum transfer = $\beta \rho QV = 1.43(1000)(37.3)(0.79) = 42,138 \text{ kg} \cdot \text{m/s}^2$

Likewise, from Equation 1.21

$$\alpha = \frac{V_1^3 A_1 + V_2^3 A_2 + V_3^3 A_3}{V^3 A} = \frac{0.5^3 (20) + 1.5^3 (16) + 0.3^3 (11)}{0.79^3 (47)} = 2.45$$

Then by using Equation 1.19

Rate of kinetic energy transfer =
$$\alpha \frac{\rho}{2} QV^2 = 2.45 \left(\frac{1000}{2}\right) (37.3)(0.79)^2 = 28,517 \text{ kg} \cdot \text{m/s}^3$$

Problem 12:

$$V = \frac{Q}{A} = \frac{Q}{y(b+my)} = \frac{100}{3.15(5+2\times3.15)} = \frac{100}{35.6} = 2.81 \text{ fps}$$

$$R = \frac{A}{P} = \frac{y(b+my)}{b+2y\sqrt{1+m^2}} = \frac{3.15(5+2\times3.15)}{5+2(3.15)\sqrt{1+2^2}} = \frac{35.6}{19.1} = 1.86 \text{ ft}$$

$$D = \frac{y(b+my)}{b+2my} = \frac{3.15(5+2\times3.15)}{5+2\times2\times3.15} = \frac{35.6}{17.6} = 2.02 \text{ ft}$$

(a) Using Equation 1.23

$$R_e = \frac{4VR}{V} = \frac{4VR}{V} = \frac{4(2.81)(1.86)}{1.217 \times 10^{-5}} = 1,925,000$$

The flow is turbulent

(b) Using Equation 1.24

$$F_r = \frac{V}{\sqrt{gD}} = \frac{2.81}{\sqrt{32.2 \times 2.02}} = 0.35$$

The flow is subcritical.

Problem 13:

- (a) Nonuniform
- (b) Nonuniform
- (c) Uniform
- (d) Nonuniform

Problem 14:

- (a) Unsteady
- (b) Steady
- (c) Steady

Problem 15:

Verify that

$$\frac{\partial (AY_C)}{\partial x} = A \frac{\partial y}{\partial x}$$

Consider a horizontal strip of flow area having a length of b and thickness of dz and extending from one side of a flow section to the other side. Suppose the vertical distance between the centroid of this strip and the channel bottom is z. Then by definition

$$\frac{\partial (AY_C)}{\partial x} = \frac{\partial}{\partial x} \int_{0}^{y} (y-z)bdz$$

Using the Leibnitz rule, we can write this as

$$\frac{\partial (AY_C)}{\partial x} = \int_0^y \frac{\partial}{\partial x} (y - z) b dz + (y - y) b dz \frac{\partial y}{\partial x} - (0 - z) b dz \frac{\partial 0}{\partial x}$$

The last two terms on the right hand side are zero. Then

$$\frac{\partial (AY_C)}{\partial x} = \int_0^y \frac{\partial}{\partial x} (y - z)bdz = \int_0^y \frac{\partial}{\partial x} (ybdz) - \int_0^y \frac{\partial}{\partial x} (zbdz)$$

Since z and b are not functions of x at a given section, the last term on the right hand side is zero. Also, since y is not a function of z and b and dz are not functions of x

$$\frac{\partial (AY_C)}{\partial x} = \int_0^y \frac{\partial}{\partial x} (ybdz) = \frac{\partial y}{\partial x} \int_0^y bdz = A \frac{\partial y}{\partial x}$$